

Surfside Beach Fishing Pier – Value Engineering (VE) Design

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STGEC 2022
Southeastern Transportation
Geotechnical Engineering Conference





8 Miles
to
Myrtle
Beach

Surfside Beach

Existing
Fishing
Pier

80 Miles to
Charleston

Fishing Pier Location

Surfside Beach, South Carolina



Owner



Damaged Fishing Pier

Post Hurricane Matthew (2016)

FEMA Grant to Town of Roughly
\$10M to replace

Original Design Conditions

Increase Deck Elevation by 10 feet

Must be concrete foundations

Difficult local geology

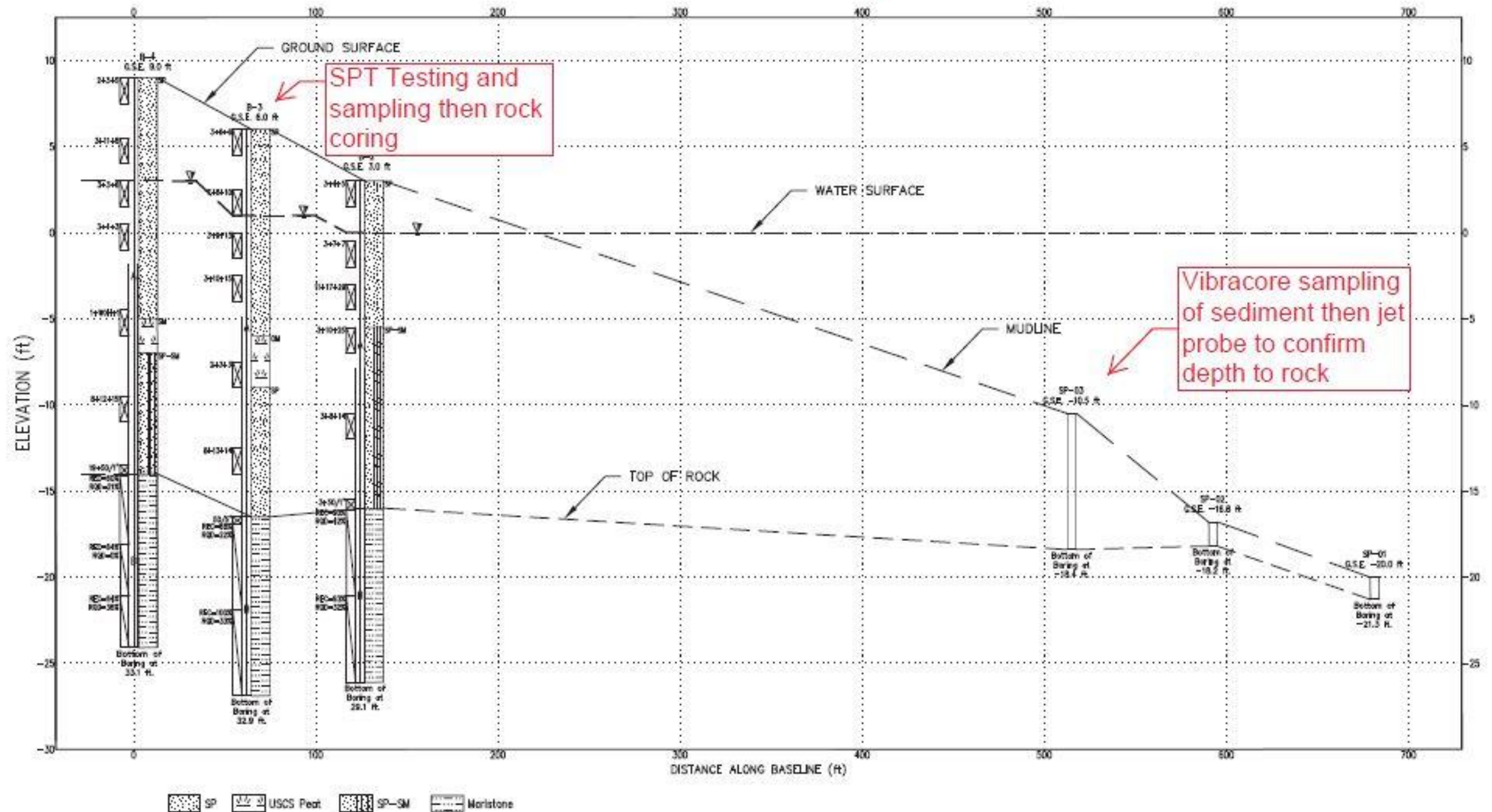
Complex wave/seismic loading

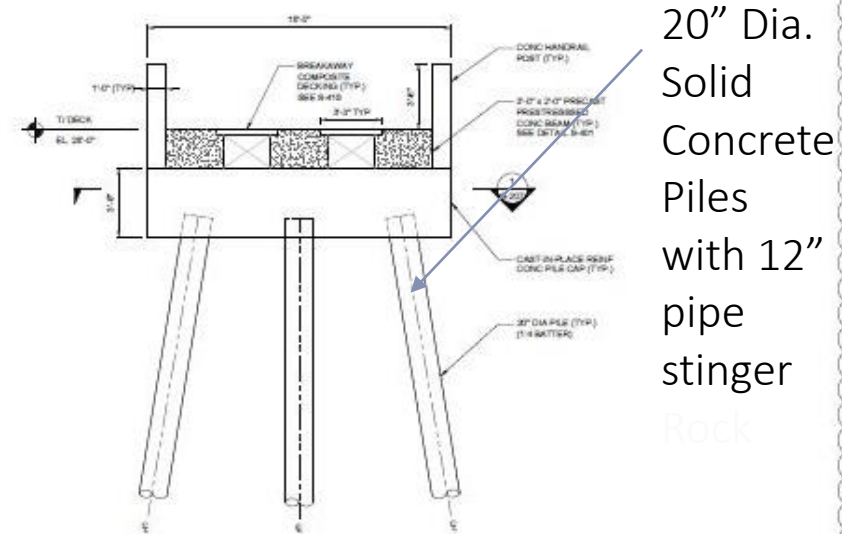
Highly corrosive marine environment

Rock
Cores from
Original
Geotech
Report



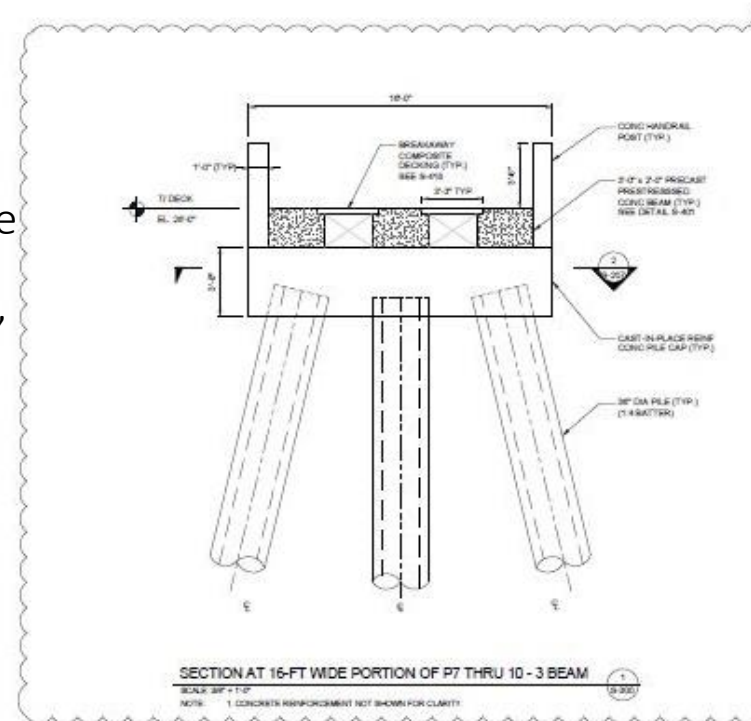
From Prebid Geotech Report





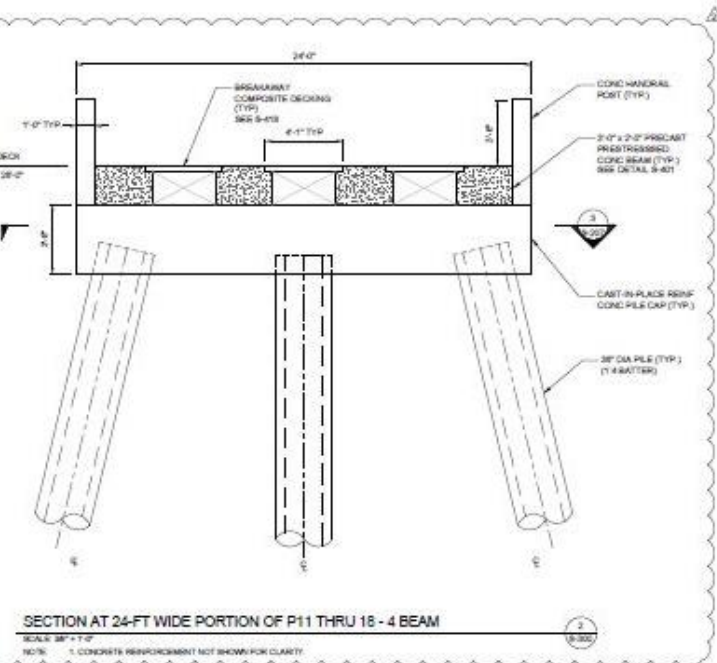
SECTION AT 16-FT WIDE PORTION OF P1 THRU P6 - 3 BEAM

SCALE: 3/8" = 1'-0"
NOTE: 1. CONCRETE REINFORCEMENT NOT SHOWN FOR CLARITY



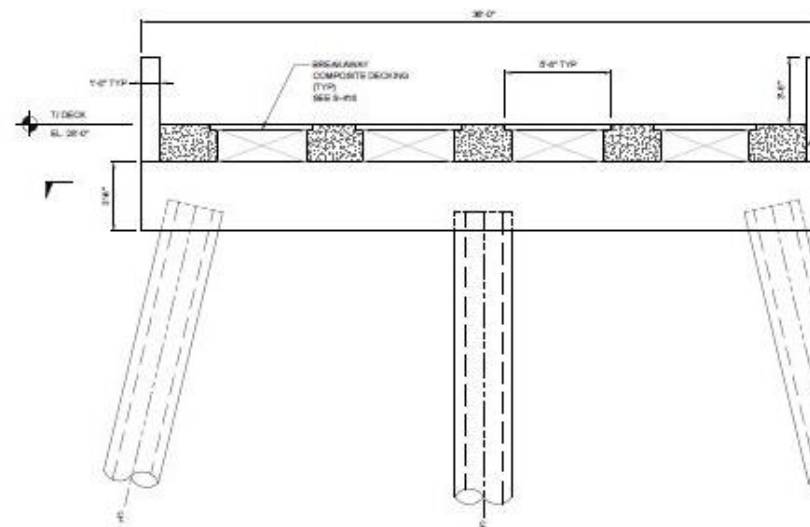
SECTION AT 16-FT WIDE PORTION OF P7 THRU 10 - 3 BEAM

SCALE: 3/8" = 1'-0"
NOTE: 1. CONCRETE REINFORCEMENT NOT SHOWN FOR CLARITY



SECTION AT 24-FT WIDE PORTION OF P11 THRU 15 - 4 BEAM

SCALE: 3/8" = 1'-0"
NOTE: 1. CONCRETE REINFORCEMENT NOT SHOWN FOR CLARITY



SECTION AT 36-FT WIDE PORTION OF P19 THRU P22 - 5 BEAM

SCALE: 3/8" = 1'-0"
NOTE: 1. CONCRETE REINFORCEMENT NOT SHOWN FOR CLARITY

Original Design Details

Battered concrete foundations

20" (P1 to P6) to 36" diameters

Drilled-in Pile (P7 to P22)

Three Piles per bent

8-foot penetration into marlstone

22 total bents

Precast Beams

Original Design Details

- Outer steel casing 2' into rock
- Clean out
- Set 36" Concrete Cylinder Pile
- Grout annulus
- Drill 8' socket into rock
- Set cage
- Concrete

Selected Construction Team



Prime Contractor



Value Engineering Structural and
Geotechnical Engineer for Fishing Pier



Foundation and Pier
Subcontractor



VE Design Conditions

Increase Deck Elevation by 10 feet

Must be concrete foundations

Difficult local geology

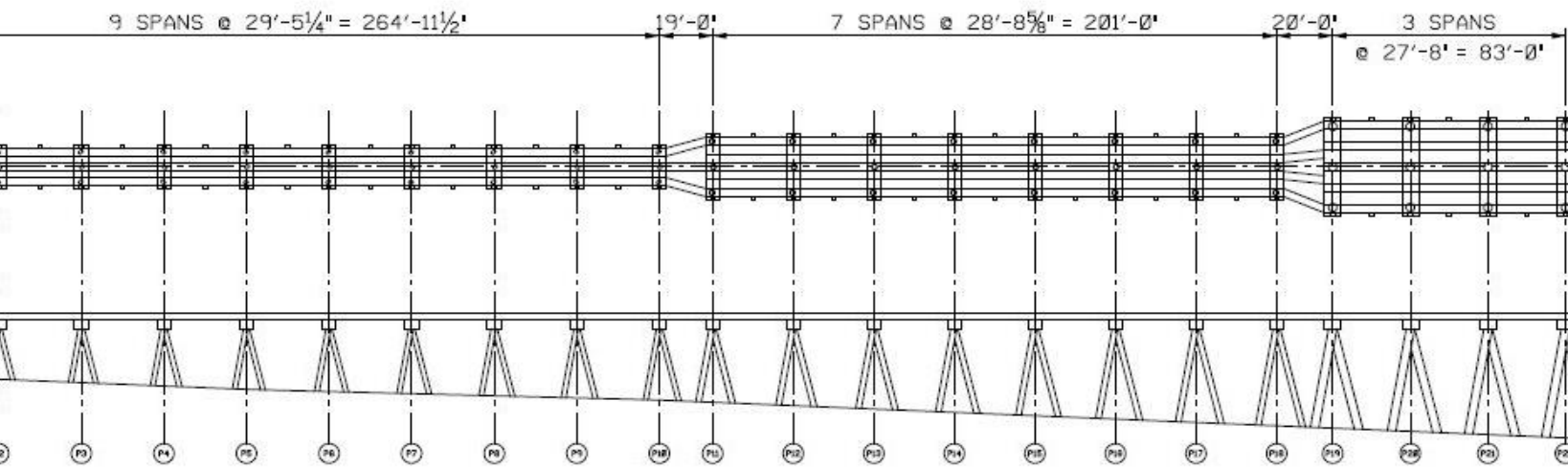
Complex wave/Seismic Loading

Highly corrosive marine environment

Same footprint as original

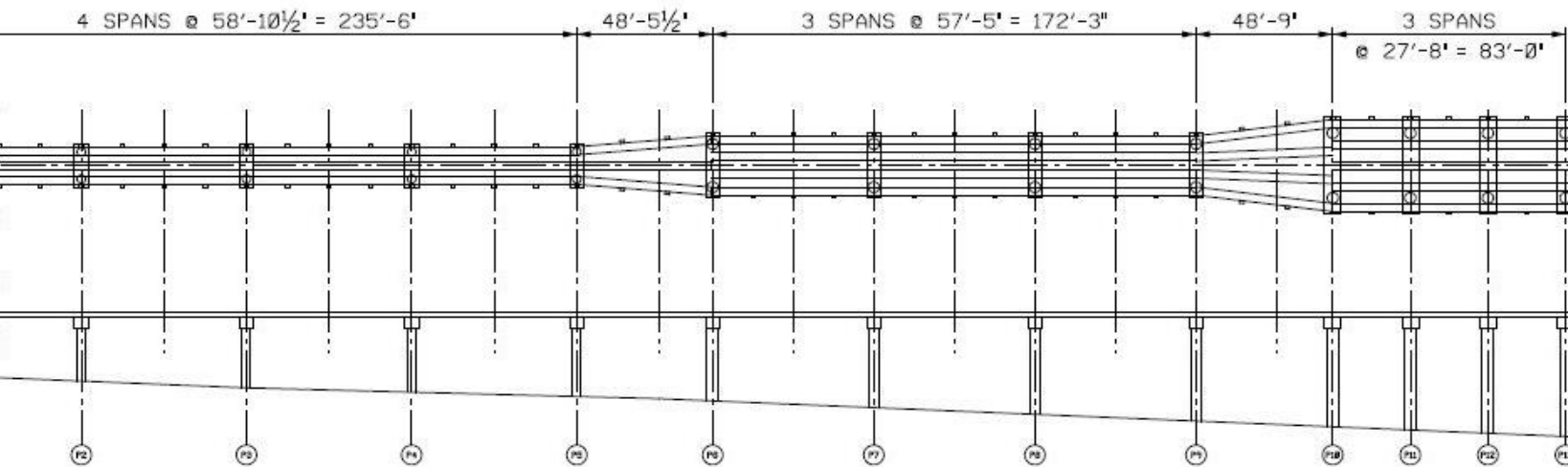
Vertical foundations

Driven foundations



Original Design

ELEVATION - AS BID PLANS



VE Design

ELEVATION - VE PROPOSAL

VE Concept

Original design (22 bents w/ 3 piles each)
and

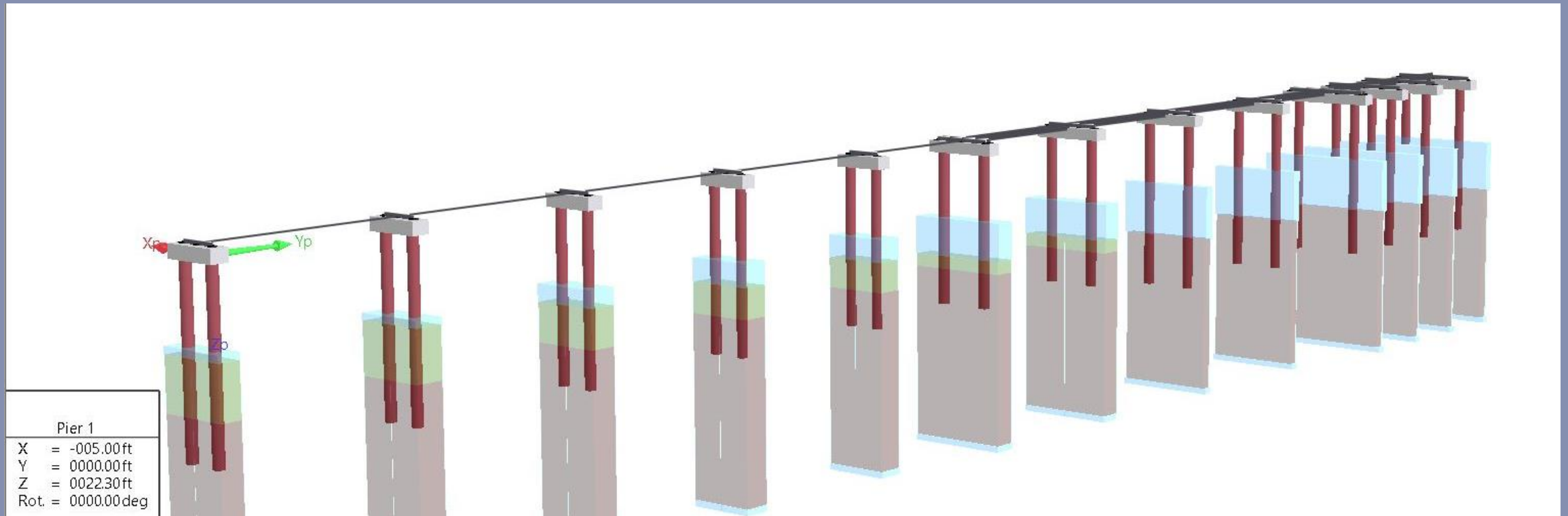
VE design (13 bents w/ 2 piles each)

~3200 LF of Pile vs ~1600 LF of Pile***

PRELIMINARY PLANS
FOR INFORMATION ONLY
DO NOT USE FOR CONSTRUCTION

ICE of
CAROLINAS, PLLC
ICE of Carolinas, PLLC
4505 Falls of Neuse Road, Suite 110
Raleigh, North Carolina 27609
Phone: 803-822-0333
License #: P-0999

JOB: SURFSIDE PIER
SUBJECT: VALUE ENGINEERING
PREPARED BY: PWG DATE: 10/10/10
CHECKED BY: _____ DATE: _____



VE 3-D Modeling

With FB-Multiplier

42" Pipe pile NO SCOUR

Hammer Information

Select from following list [10/17/2016-2003]:

ID: 583

ID	Name	Type	Ram W/Ecc. M.	Energy/Power
582	APE D 125-42	OED	27.563	310.084
583	APE D 50-42	OED	11.025	124.031
584	APE D 12-42	OED	2.646	29.767

Hammer parameters

Efficiency **0.8**
Pressure **1160** psi Fixed **73** %
Stroke **11.25** ft Variable

Pile material

☐ Concrete ☒ Steel ☐ Timber

Cushion Information

	Hammer	Pile
Area	491.	0. in ²
Elastic Modulus	285.	0. ksi
Thickness	3.5	0. in
C.O.R.	0.8	0.
Stiffness	0.	0. kips/in
Helmet Weight	7.	0. kips

Pile Information

Length	65. ft	Auto	Segments
Penetration	30. ft	Auto.	S-Length
Section Area	128.8 in ²	Auto.	S-St, Wt
Elast Modulus	29000. ksi	0	Splices
Spec Weight	492.0 lb/ft ³		
Toe Area	128.8 in ²	Pile Type:	
Perimeter	10.995 ft	Pipe	
Pile Size	42. in		

Ultimate Capacities (up to 10)

kips	
1	500.0
2	568.0
3	611.0
4	800.0
5	950.0
6	1100.0
7	1250.0
8	1400.0
9	1550.0
10	1700.0

Incr. **0** Action >>

Soil Parameters

2nd Toe - No

Quake

Shaft **0.1** in Const
Toe **0.1** in

Damping

Shaft **0.13** s/ft Const
Toe **0.15** s/ft Smith

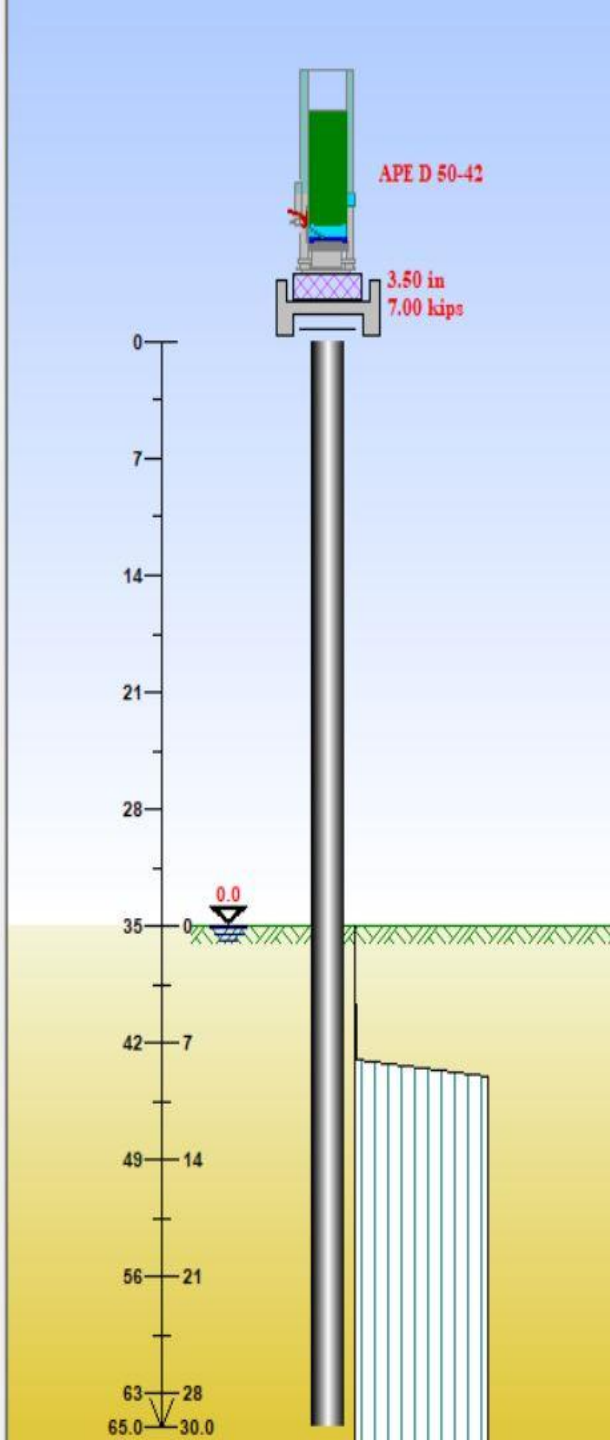
Shaft Resistance

Percentage **95** %

Dist. Shape Num **0.0**

Residual Stress Analysis:

No



VE Constructability

Model and properly size contractors' equipment with GRLWEAP

VE Design Details

Vertical foundations

Steel foundations

36 to 42" diameters

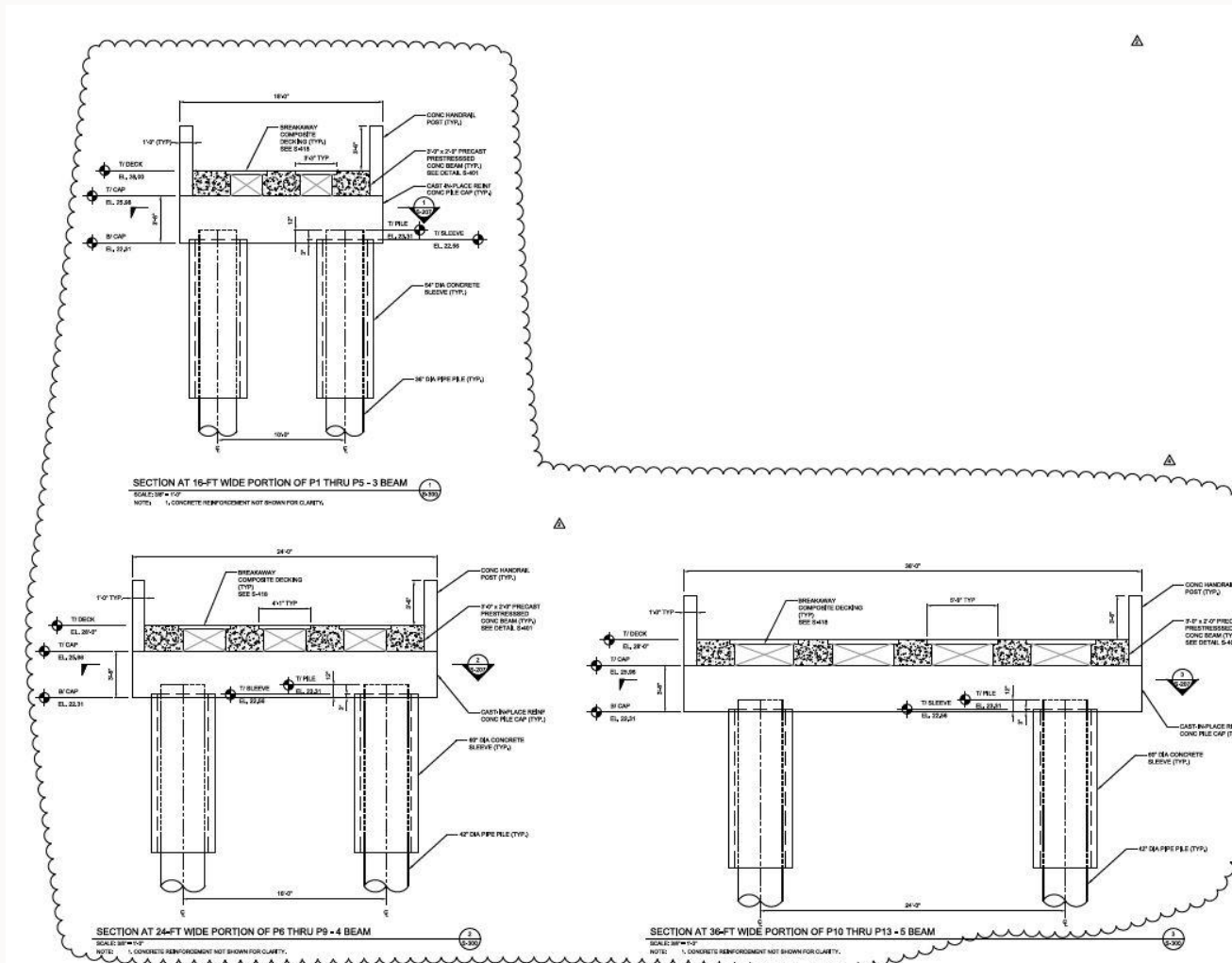
Driven pile (contingency drilling)

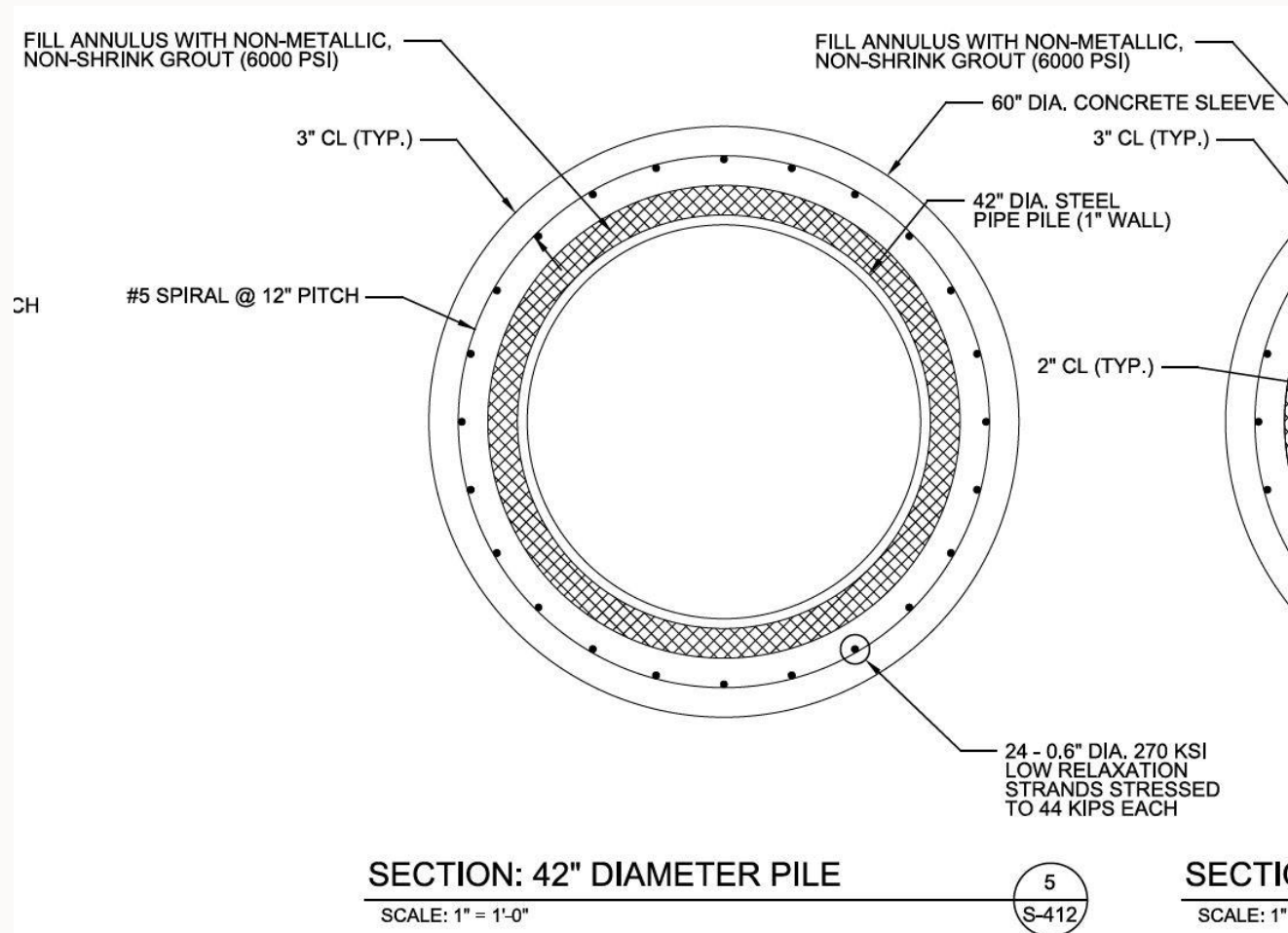
Two piles per bent

Up to 22-foot penetration into marlstone

13 total bents

Similar Precast Beams





VE Design Details

Steel Piles w/ corrosion protection coatings

Concrete sleeve

Grout corrosion protection

PIER	AXIAL-CAPACITY (ASD)	MIN PILE TIP EL (FT)
1 TO 5	235 KIPS	-31
6 TO 7	303 KIPS	-35
8 TO 9	284 KIPS	-37
10 TO 13	284 KIPS	-40

PILE TABLE OF VARIABLES							
PILE CAP NO.	MUDLINE EL. (FT.)	PILE DIA. (IN.)	SLEEVE DIA. (IN.)	LP (FT.)	EL. P (FT.)	LS (FT.)	EL. S (FT.)
P1	4	36	54	55	-31	35	-12
P2	2	36	54	55	-31	37	-14
P3	-1	36	54	55	-31	39	-16
P4	-2	36	54	55	-31	39	-16
P5	-3	36	54	55	-31	39	-16
P6	-5	42	66	59	-35	39	-16
P7	-7	42	66	59	-35	39	-16
P8	-11	42	66	61	-37	41	-18
P9	-15	42	66	61	-37	41	-18
P10	-20	42	66	64	-40	44	-21
P11	-20	42	66	64	-40	44	-21
P12	-20	42	66	64	-40	44	-21
P13	-20	42	66	64	-40	44	-21

P = Pipe Pile

S = Sleeve

VE Design Details

Plan Axial Capacities

Plan Min. Pile Tip Elev.

Bent/Pier 1-5 36" Piles

~15' min Embedment into Marlstone

Bent/Pier 6-13 42" Piles

~19-22' min. embedment into Marlstone

Drive to FS of 2.0 with PDA Testing/WEAP

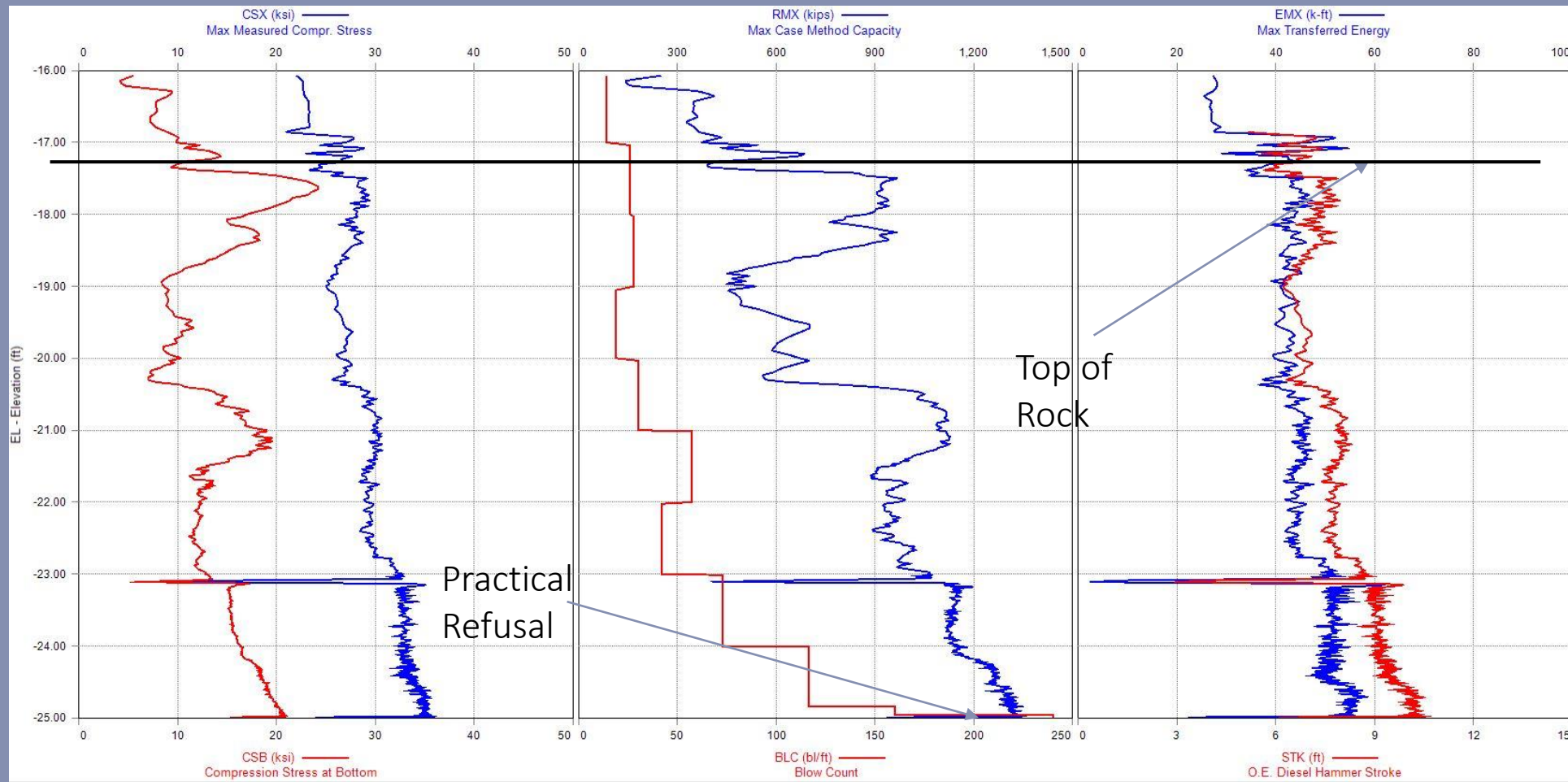
Predrill if can't be installed by driving



Approx.
Layout of
Fishing Pier
Bents

Summer 2021 – VE Design Ongoing

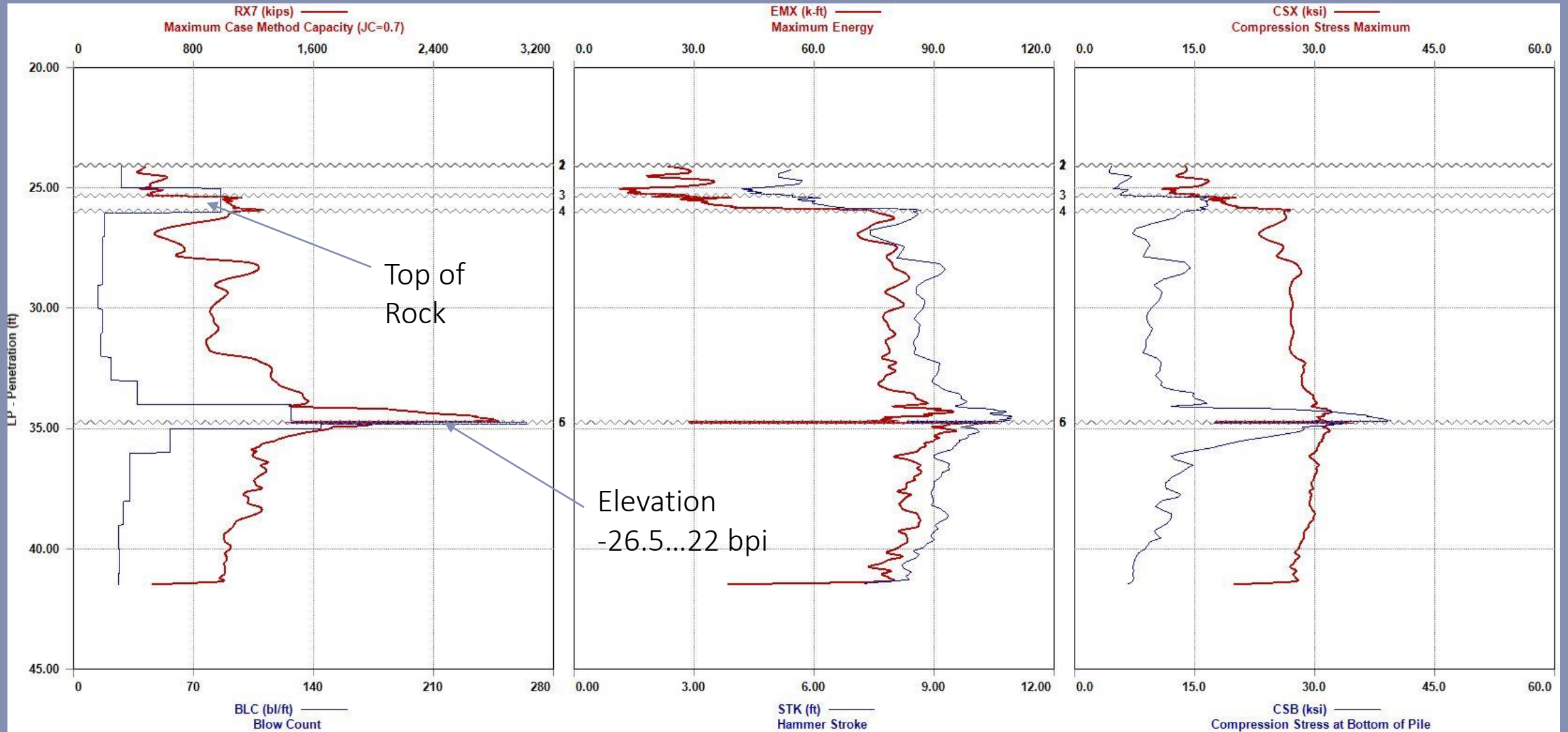
Picture Courtesy of Surfside Beach
Oceanfront Hotel Webcam



PP30x0.529
APE D50-42

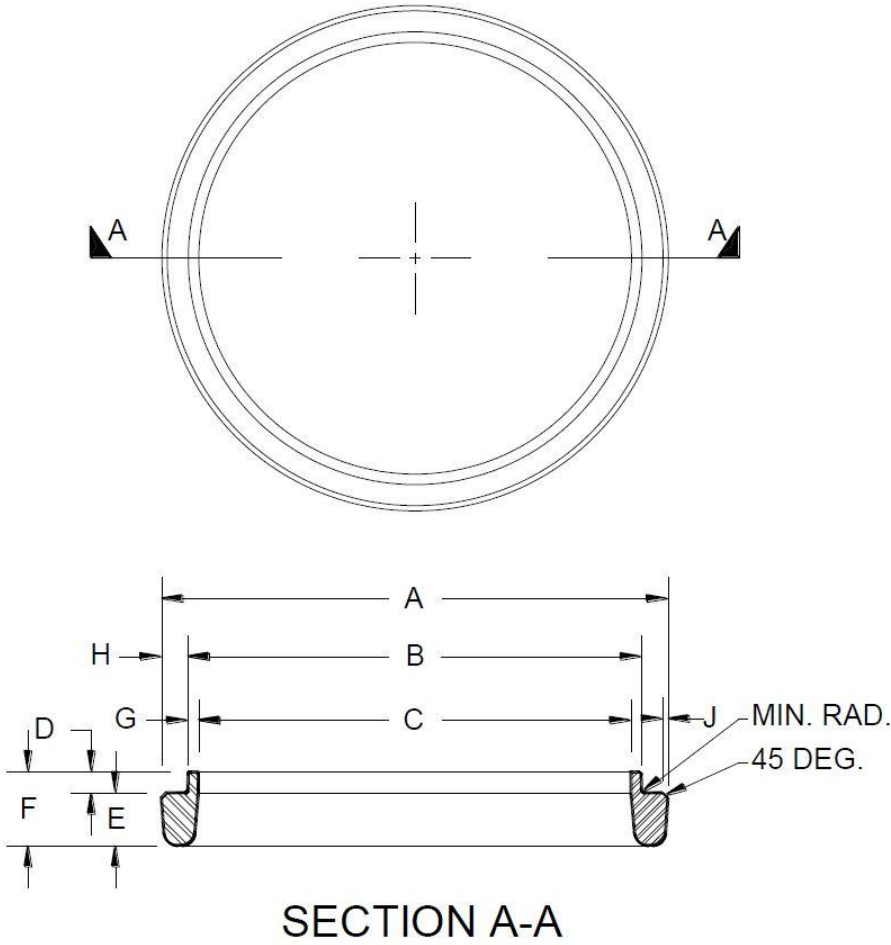
PDA Results First Trestle Pile

PP36x0.741 with outside fit shoe
APE D80-42

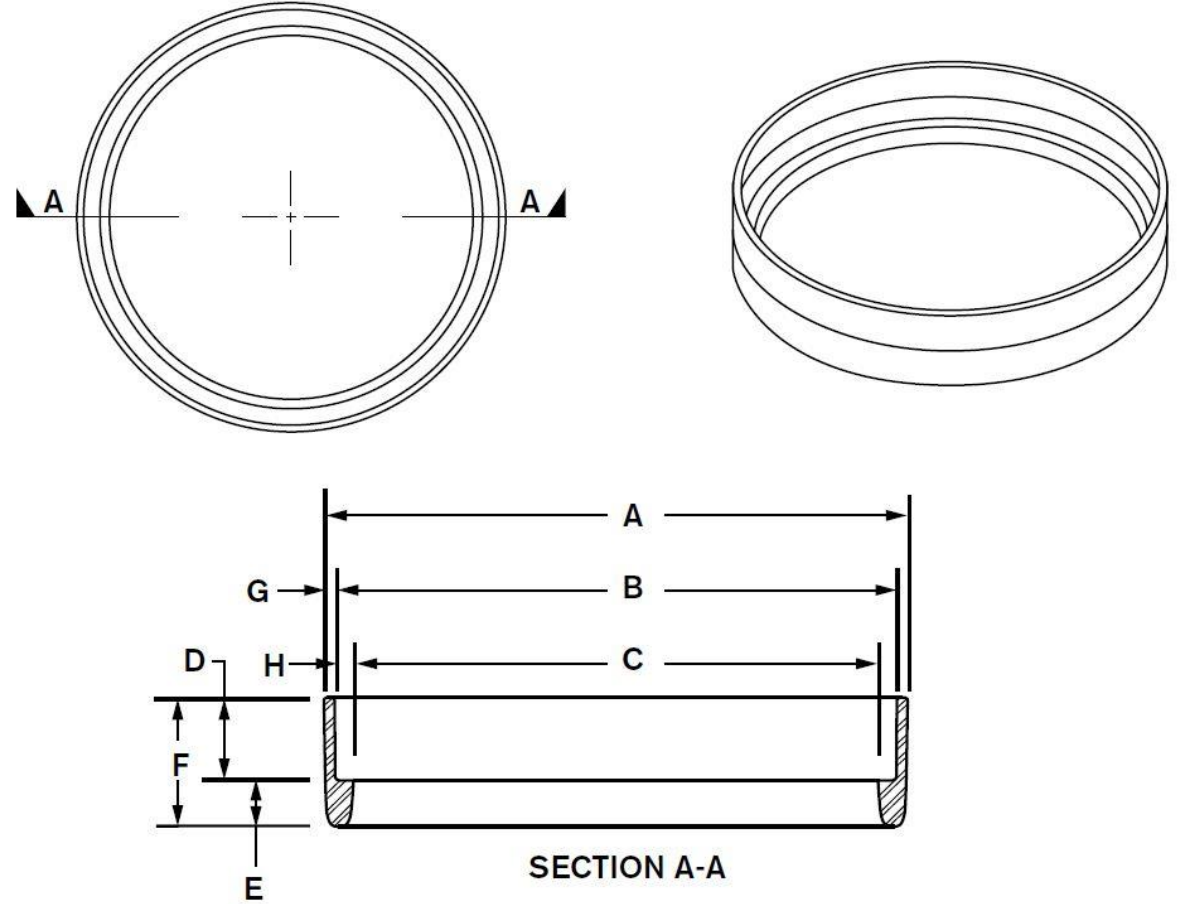


PDA Results First Production Pile (11/17/21)

Inside Fit (42" Pipes Bents 7 to 13)



Outside Fit (36" pipes Bent 1 and 3 to 6)



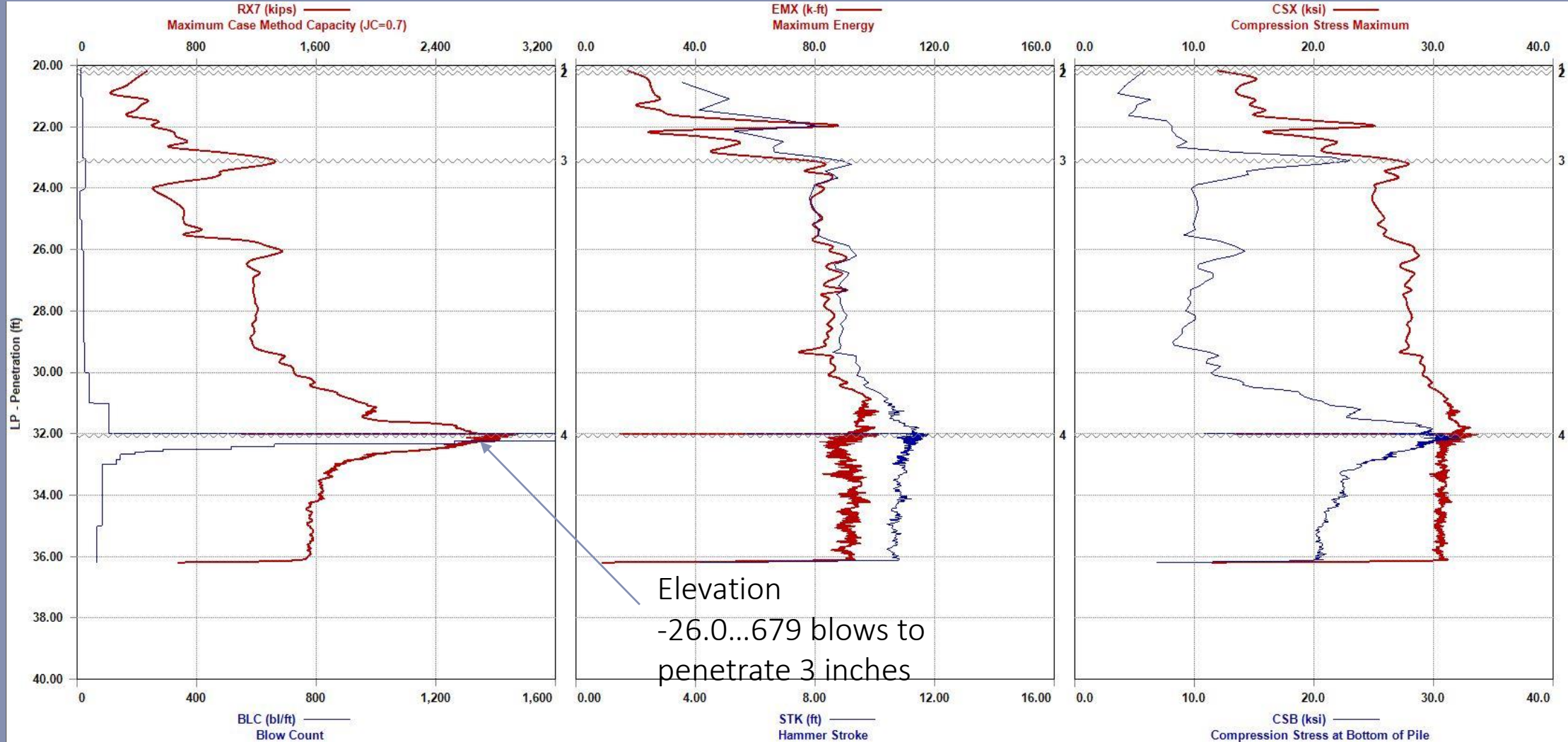
Pipe Pile Cutting Shoes



Pipe Pile Cutting Shoes

PP36x0.741 with no cutting shoe

APE D80-42



PDA Results Bent 2 (No shoes)



Current Progress

September 2022



Current Progress

September 2022



Current Progress

September 2022



Current Progress

September 2022



Current Progress

September 2022

- 4-6 piles remaining
- Relatively easy driving (<50 bpf) barring thin hard layer at Elevation -26 to -27 ft at near practical refusal
- No contingency drilling needed
- Likely pile overrun at last 4 bents to meet axial load (like end of work trestle)
- Did do long term restrike (1 week) at end of work trestle, little to no setup

Current Progress

September 2022

Current Progress

Hurricane Ian (9/30/22)

Picture Courtesy of Surfside Beach
Oceanfront Hotel Webcam



- Cost Savings? Likely Yes**
- Time Savings? Yes
- Better aesthetics? Maybe
- Did we add value??

Overall Original
Design vs. VE

Questions?